SOUTH SAN JOAQUIN IRRIGATION DISTRICT LONG TUNNEL MAINTENANCE **UPSTREAM PORTAL - SHOTCRETE SHORING WALL**







SCOPE OF WORK

- WORK CONSISTS OF CONSTRUCTING A SHOTCRETE SHORING WALL ON THE SOUTH SIDE OF THE OAKDALE IRRIGATION DISTRICT AND SOUTH SAN JOAQUIN IRRIGATION DISTRICT JOINT SUPPLY CANAL NEAR THE LONG TUNNEL UPSTREAM PORTAL.
- WORK INCLUDES, BUT IS NOT LIMITED TO:
- PERFORM SAFETY SCALING OF THE EXISTING LINSUPPORTED ROCK SLOPE
- REMOVE ROCK AND SOIL RUBBLE IN THE CANAL AND PLACE AS GENERAL STOCKPILE FILL IN THE DISPOSAL AREA AS SHOWN ON THE DRAWINGS.
- REMOVE VEGETATION FROM AREAS TO RECEIVE SHOTCRETE AND DISPOSE DESITE
- PREPARE GROUND SURFACES FOR SHOTCRETE PLACEMENT AND INSTALL DRAIN STRIPS.
- PLACE FIRST 3-INCH FIBER-REINFORCED SHOTCRETE LAYER DRILL AND INSTALL CEMENT-GROUTED ROCK DOWELS
- PLACE SECOND 3-INCH FIBER-REINFORCED SHOTCRETE LAYER

THE WORK AREA IS IN THE VICINITY OF KNIGHTS FERRY. CALIFORNIA - SOUTH SAN JOAQUIN IRRIGATION

OWNER'S CONTACT: SOUTH SAN JOAQUIN IRRIGATION DISTRICT RIPON. CALIFORNIA 95366 FORREST KILLINGSWORTH fkillingsworth@ssiid.com

CONSULTING GROUP (P&P) 19969 GREENLEY ROAD SONORA, CALIFORNIA 95370

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THE GROUND TOPOGRAPHY SURVEY (GT) SHOWN WAS COLLECTED BY ERIK OHLSON & ASSOCIATES DATED: JUNE 14, 2023, FILE: 03305B Long Tunnel dwg, HORIZONTAL AND VERTICAL DATUM NAD 83 CALIFORNIA STATE PLANE ZONE 3 AND NAVD88, RESPECTIVELY.

- THE CONTRACTOR SHALL PREPARE STAGING AND LAYDOWN AREAS FOR THE PROJECT AT THE ALLOWABLE LOCATIONS SHOWN ON THESE DRAWINGS AND ACCORDING TO THE CONTRACT DOCUMENTS. IF A DIFFERENT AREA IS DEEMED NECESSARY, THE CONTRACTOR SHALL COORDINATE WITH THE ENGINEER AND OWNER PRIOR TO MOBILIZATION.
- THE CONTRACTOR IS RESPONSIBLE FOR MAINTAINING DISTRICT ACCESS ROADS DURING CONSTRUCTION. ANY DAMAGE OR ALTERATION TO THE DISTRICT'S ACCESS ROADS SHALL BE REESTABLISHED BY THE CONTRACTOR AT THE DISCRETION OF THE ENGINEER.
- THE CONTRACTOR SHALL KEEP SITE AND CANAL ACCESS ROUTES CLEAR OF DEBRIS, MATERIALS AND EQUIPMENT AT ALL TIMES.
- THE CONTRACTOR AND ALL OTHER PROJECT PERSONNEL SHALL KEEP THE GATES CLOSED AND
- THE MAXIMUM SPEED LIMIT IS 15 MPH ALONG ON-SITE ACCESS ROADS. CONTRACTOR SHALL POST
- REMOVE ALL MATERIALS AND EQUIPMENT FROM CANAL WHEN CONTRACTOR IS NOT ON-SITE

GENERAL

- ALL WORK SHALL TAKE PLACE WITHIN 50-FT. OF THE CANAL CENTERLINE, WHICH IS THE OWNER'S
- THE CONTRACTOR SHALL FURNISH ALL NECESSARY LABOR, MATERIALS, SUPPLIES, AND EQUIPMENT FOR CONSTRUCTION OF THE IMPROVEMENTS SHOWN IN THE CONTRACT DOCUMENTS
- IT IS INTENDED THAT THESE PLANS. SPECIFICATIONS AND CONTRACT DOCUMENTS INCLUDE ALL LABOR AND MATERIALS NECESSARY AND PROPER FOR THE PROPOSED WORK / IMPROVEMENTS AND THAT THE WORK BE COMPLETED IN ACCORDANCE WITH THEIR TRUE INTENT AND PURPOSE THE CONTRACTOR SHALL NOTIFY THE ENGINEER IMMEDIATELY REGARDING ANY DISCREPANCIES AND AMBIGUITIES WHICH MAY EXIST IN THE PLANS AND SPECIFICATIONS
- IF THE PLANS OR SPECIFICATIONS DESCRIBE PORTIONS OF THE WORK IN GENERAL TERMS, BLIT PREVAIL AND THAT ONLY MATERIAL AND WORKMANSHIP OF THE FIRST QUALITY ARE TO BE USED
- THE OWNER SHALL RETAIN A CONSTRUCTION MATERIALS TESTING AND INSPECTION AGENCY (TESTING AGENCY) FOR THE REQUIRED TESTING AND INSPECTIONS. THE CONTRACTOR SHALL PROVIDE THE OWNER. THE ENGINEER (PROVOST & PRITCHARD), AND THE TESTING AGENCY AT LEAST TWO WORKING DAYS NOTICE PRIOR TO STARTING WORK
- WORKMANSHIP AND MATERIALS SHALL CONFORM TO THE REQUIREMENTS IN THE LATEST VERSIONS OF THE CALIFORNIA BUILDING CODE (CBC), CALTRANS STANDARD SPECIFICATIONS AND THE SOUTH SAN JOAQUIN IRRIGATION DISTRICT STANDARD SPECIFICATIONS AND DRAWINGS.
 WORKMANSHIP AND MATERIALS SHALL CONFORM TO THE REQUIREMENTS OF STANISLAUS COUNTY AND GOOD TRADE PRACTICE
- THE CONTRACTOR SHALL BE APPROPRIATELY LICENSED BY THE STATE OF CALIFORNIA TO PERFORM THE WORK SHOWN/DESCRIBED IN THE CONSTRUCTION DOCUMENTS AND SHALL BE EXPERIENCED IN THESE TYPES OF WORK.
- THE CONTRACTOR SHALL SECURE NECESSARY PERMITS FROM AGENCIES, AS NECESSARY
- THE CONTRACTOR SHALL NOTIFY UNDERGROUND SERVICE ALERT (USA) BEFORE STARTING EARTHWORK, DRILLING GROUND, AND OTHER EXCAVATION WORK, AND THEY SHALL NOT BEGIN EXCAVATION UNTIL THE USA-INDICATED EARLIEST START DATE AND TIME
- THE CONTRACTOR SHALL RENEW USA TICKETS BEFORE THEY EXPIRE AND MAINTAIN UNTIL COMPLETION OF DRILLING AND EXCAVATION WORK
- THE CONTRACTOR IS RESPONSIBLE FOR VERIFYING EXISTING UTILITIES IN THE FIELD. COSTS OF REPAIRING ANY DAMAGES OR INJURIES CAUSED BY THE CONTRACTOR SHALL BE BORNE BY THE
- 12. THE CONTRACTOR SHALL KEEP EXISTING UTILITIES IN SERVICE AT ALL TIMES OR COORDINATE REQUIRED UTILITY SHUT-DOWNS WITH THE OWNER AND THE UTILITY OWNERS
- THE CONTRACTOR SHALL PROTECT EXISTING MONUMENTS, FACILITIES AND IMPROVEMENTS FROM DAMAGE RESULTING FROM CONTRACTOR'S WORK. ANY DAMAGE CAUSED BY CONTRACTOR SHALL BE REPAIRED AT THE CONTRACTOR'S EXPENSE. ANY EXISTING MONUMENTS THAT MUST BE REMOVED DURING THE WORK SHALL BE REPLACED BY THE CONTRACTOR
- THE CONTRACTOR SHALL REVIEW THE WORK SCOPE AND NOTIFY THE OWNER AND PROVOST &
- 15 THE CONTRACTOR IS SOLELY AND COMPLETELY RESPONSIBLE FOR JOB SITE CONDITIONS DURING THE COURSE OF CONSTRUCTION FOR THIS PROJECT, INCLUDING SAFETY OF PERSONS AND PROPERTY. THIS REQUIREMENT SHALL APPLY CONTINUOUSLY AND SHALL NOT BE LIMITED TO NORMAL WORKING HOURS.
- METHODS, TECHNIQUES, SEQUENCES, OR PROCEDURES FOR SAFETY PRECAUTIONS.
- SHOULD ANY CONTRACTOR OR SUBCONTRACTOR FIND DEFICIENCIES, ERRORS, CONFLICTS OR OMISSIONS IN THESE CONTRACT DRAWINGS OR THE CONTRACT SPECIFICATIONS OR SHOULD THERE BE ANY DOUBT AS TO THEIR MEANING OR INTENT. THE CONTRACTOR SHALL IMMEDIATELY
- WRITTEN DIMENSIONS TAKE PRECEDENCE OVER SCALED DIMENSIONS. IF THERE IS A CONFLICT THEN NOTIFY THE ENGINEER TO OBTAIN A CLARIFICATION. NO DEVIATION OR SUBSTITUTION SHALL BE ALLOWED WITHOUT OBTAINING WRITTEN APPROVAL FROM THE ENGINEER.
- BOUNDARY LINES SHOWN ON THESE DRAWINGS ARE APPROXIMATE AND ARE FOR REFERENCE
- 20. THE DESIGN IS LIMITED TO THE WORK DESCRIBED IN THE CONTRACT DOCUMENTS. THESE
- ALL USERS OF THESE DRAWINGS AGREE TO HOLD PROVOST & PRITCHARD HARMLESS FOR ANY AND ALL WORK THAT DOES NOT CONFORM TO REQUIREMENTS AND MINIMUM STANDARDS OF THE RELEVANT AND CURRENT BUILDING CODE, LOCAL ORDINANCES, AND ACCEPTABLE STANDARDS
- 22. PROVOST & PRITCHARD ASSUMES NO RESPONSIBILITY FOR THE PERFORMANCE OF PRODUCTS OR
- 23. THE CONTRACTOR SHALL DISPOSE ALL DEMOLISHED MATERIALS, VEGETATION, EXCESS CONSTRUCTION MATERIALS, AND OTHER WASTE LEGALLY AT AN OFFSITE LOCATION

THE CONTRACTOR IS RESPONSIBLE FOR CONSTRUCTING MAINTAINING OPERATING AND REMOVING TEMPORARY STORM WATER FACILITIES, MANAGING STORM WATER FLOWS, AND PREPARING AND IMPLEMENTING THE STORM WATER POLLUTION CONTROL PROGRAM (WPCP)





FOOT

GALLON

INVERT

POUNDS

GAL

HMA

INV

LBS

GRADE BREAK

HOT MIX ASPHALT

INSIDE DIAMETER

DETAIL IDENTIFICATION



WIDTH

WWE WELDED WIRE FARRIC

REVISION NUMBER

ABBREVIATIONS

	ADDREVIATIO	INO	
В	AGGREGATE BASE ROCK	LCC	LOW DENSITY CELLULAR
C	ASPHALTIC CONCRETE		CONCRETE
.D	AREA DRAIN	LF	LINEAR FEET
VE	AVERAGE	MAX	MAXIMUM
F	BOTTOM OF FOOTING	MH	MANHOLE
HC	BEGIN HORIZONTAL CURVE	MIN	MINIMUM
K	BOTTOM OF KEY	(N)	NEW
VC	BEGIN VERTICAL CURVE	OC	ON CENTER
W	BOTTOM OF WALL	OG	ORIGINAL GROUND
DF	CONTROLLED DENSITY FILL	(P)	PROPOSED
IP	CAST-IN-PLACE	P.C.	POINT OF CURVATURE
PP	CORRUGATED PLASTIC PIPE	P.C.C.	POINT OF COMPOUND
	CENTER LINE		CURVATURE
MP	CORRUGATED METAL PIPE	P.I.	POINT OF INTERSECTION
PP	CORRUGATED PLASTIC PIPE	PIP	PROTECT IN PLACE
ONC	CONCRETE	P.R.C.	POINT OF REVERSE CURVATUR
Υ	CUBIC YARD	P.T.	POINT OF TANGENCY
1	DROP INLET	PRF	PAVEMENT REINFORCING FAB
IA	DIAMETER	PSI	POUNDS PER SQUARE INCH
i	DIAMETER	PVC	POLYVINYL CHLORIDE PIPE
L	DESIGN LOAD	P.W.	PROCESS WASTE
Ξ)	EXISTING	R	RADIUS
É	EACH FACE	RCP	REINFORCED CONC. PIPE
G	EXISTING GROUND	S	SLOPE
HC	END HORIZONTAL CURVE	SCH	SCHEDULE
L	ELEVATION	SD	STORM DRAIN
LEC	ELECTRICAL	SF	SQUARE FEET
S	EACH SIDE	SG	SUBGRADE
VC	END VERTICAL CURVE	SS	SANITARY SEWER
W	EACH WAY	STA	STATION
TW	EDGE OF TRAVELED WAY	SWPPP	STORM WATER POLLUTION
DR	FULL DEPTH RECLAMATION		PREVENTION PLAN
F	FINISH FLOOR GRADE	TC	TOP OF CONCRETE
G	FINISH GRADE	TBD	TO BE DETERMINED
L	FLOW LINE	TF	TOP OF FOOTING
ОС	FACE OF CONCRETE	TYP	TYPICAL
s	FINISH SURFACE	TW	TOP OF WALL
RC	FIBER REINFORCED CONCRETE	U.N.O.	UNLESS NOTED OTHERWISE
RS	FIBER REINFORCED SHOTCRETE	V.I.F.	VERIFY IN FIELD



SITE LOCATION

DRAWING INDEX						
SHEET NO.	TITLE					
1.0	COVER SHEET AND NOTES					
1.1	NOTES					
2.0	OVERALL SITE PLAN					
2.1	SHOTCRETE SHORING WALL PLAN					
2.2	LAYDOWN AREA					
3.0	SHOTCRETE SHORING WALL ELEVATION					
3.1	SHOTCRETE SHORING WALL SECTIONS					
4.0	SHOTCRETE SHORING WALL DETAILS					



LONG TUNNEL

ANDREW KOSITSKY GF 2532

DRAFTED BY: CHECKED BY

JOB NO: 1055-23005 ROJECT NO: 23

PHASE: 005

INCH. ADJUST SCALE FOR REDUCED OR ENLARGED PLANS

EARTHWORK:

- SOILS AND ROCK DEBRIS SHALL BE PLACED IN THE ROCK/SOIL DISPOSAL AREA AS UNCOMPACTED
- THE CONTRACTOR SHALL VISUALLY INSPECT THE SITE TO DETERMINE THE EXTENT OF SITE
- THE CONTRACTOR SHALL REMOVE TREES AND VEGETATION. CLEAR AND GRUB AT LEAST 2-FEET

SHOTCRETE SHORING WALL

- SPECIAL INSPECTIONS ARE REQUIRED. SEE INSPECTIONS TABLE ON THIS SHEET.
- 2. SHOTCRETE WALLS SHALL HAVE A GUN FINISH

SHOTCRETE

- REFER TO CONTRACT SPECIFICATION SECTION 03470 FOR SHOTCRETE.
- THE CONTRACTOR SHALL SUBMIT SHOTCRETE MIX DESIGNS TO PROVOST & PRITCHARD FOR
- DIMENSIONS SHOWN IN DETAILS ARE TYPICAL AND ARE MINIMUMS WHERE NOTED.
- 4. SHOTCRETE SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH OF 4.000 PSI AT 28 DAYS.
- ADD SYNTHETIC FIBER TO SHOTCRETE AT A RATE OF 4 LBS/CU. YD. PER MANUFACTURER'S
- ADD FLY ASH OR SLAG AT A RATE OF 10% TO 25% OF PORTLAND CEMENT BY WEIGHT.
- THE ROCK AND SOIL SURFACES TO RECEIVE SHOTCRETE SHALL BE THOROUGHLY CLEANED OF ALL VEGETATION AND DEBRIS BEFORE RECEIVING SHOTCRETE. THESE SURFACES SHALL BE WETTED AND DAMP BEFORE SHOTCRETE IS PLACED.
- INSPECTION AND ACCEPTANCE BY ENGINEER OF SUBGRADES, SUBSURFACE DRAINAGE, REINFORCEMENT AND SHOTCRETE IS REQUIRED
- SHOTCRETE SHALL HAVE NO CONTRACTION JOINTS. THE FINISHES SHALL CONFORM TO PROJECT SHOTCRETE SPECIFICATIONS, OR AS DIRECTED BY PROVOST & PRITCHARD OR THE OWNER'S REPRESENTATIVE.

SPECIAL INSPECTIONS ARE REQUIRED. SEE INSPECTIONS TABLE

SCHEDULE/PROCEDURES, AND ACCEPTANCE CRITERIA.

- THE CONTRACTOR SHALL INSTALL ROCK DOWELS ACCORDING TO SPECIFICATION SECTION 02460, OR AS DIRECTED BY P&P
- 3. THE CONTRACTOR SHALL PERFORM ROCK DOWEL LOAD TESTS ON SACRIFICIAL ROCK DOWELS.
- SEE LOAD TEST NOTES FOR REQUIRED TEST FREQUENCY, TEST REQUIREMENTS, LOAD TEST
- BARS FOR PRODUCTION ROCK DOWELS SHALL BE GRADE 75, #8 ALL-THREAD, AND EPOXY COATED
- OR GALVANIZED.
- S. ROCK DOWEL DRILL HOLES SHALL BE 4 INCHES IN DIAMETER MINIMUM. DRILL HOLES FOR TEST DOWELS SHALL BE 4 INCHES IN DIAMETER.
- THE DESIGN COMPRESSIVE STRENGTH OF CEMENT GROUT FOR ROCK DOWELS IS 4,000 PSI AT 28
- 8. THE GROUT MIX SHALL NOT EXCEED 4.75 GALLONS OF WATER PER SACK OF CEMENT (94 LBS). IF THE CONTRACTOR CAN PUMP A STIFFER MIX, THEN THEY SHALL USE 4.5 GALLONS OF WATER PER SACK OF CEMENT.
-). THE CONTRACTOR SHALL DRILL ROCK DOWELS NEARLY PERPENDICULAR IN PLAN VIEW TO THE SHOTCRETE FACE
- 10. BARS SHALL BE FULLY CEMENT GROUTED.

ROCK DOWEL LOAD TESTS

- SPECIAL INSPECTIONS ARE REQUIRED. SEE INSPECTION NOTES.
- 2. ROCK DOWEL TESTING SHALL CONFORM TO SPECIFICATIONS SECTION 02460 OR AS DIRECTED BY
- 3. ROCK DOWEL LOAD-TEST PROGRAM SHALL INCLUDE:
- PROOF/VERIFICATION TEST AT LOCATIONS SELECTED BY P&P.
- FREQUENCY: AT LEAST ONE TEST FOR EVERY 20 PRODUCTION DOWELS.
- 4. TEST DOWELS SHALL CONSIST OF GRADE 75, #8 BARS
- 5. JACK CAPACITY SHALL BE 30 TONS OR OTHERWISE APPROVED BY ENGINEER.
- 6. CAUTION: DO NOT OVERLOAD TEST DOWELS.
- ALLOWABLE LOAD IS 90% OF THE YIELD STRENGTH
- THE MAXIMUM ALLOWABLE LOAD FOR GRADE 75 #8 BAR IS 53 KIPS.
- 7. MIN. UN-BONDED LENGTH (TEST DOWELS) = 2 FEET.

8. LOAD INCREMENTS AND LOAD TIME SCHEDULE

OAD INCREMENT	HOLD TIME		
AL.	UNTIL STABLE		
.25TL	UNTIL STABLE		
.50TL	UNTIL STABLE		
.75TL	UNTIL STABLE		
.00TL	UNTIL STABLE		
.25TL	UNTIL STABLE		

1.50TL (CREEP TEST LOAD) 10 OR 60 MIN. (SEE BELOW)

1.75TL UNTIL STABLE 2.00TL (MAX TEST LOAD) 10 OR 60 MIN. (SEE BELOW)

UNTIL STABLE UNTIL STABLE OR PULLOUT

MAX BAR LOAD OR PULLOUT (VERIFICATION TEST)

AL (IF NO PULLOUT)

(TL IN POUNDS = TEST LOAD = [3.14 X HOLE DIA. IN INCHES X BONDED LENGTH IN INCHES) X DESIGN BOND STRESS IN PSI] SEE NOTE 9)

(AL = ALIGNMENT LOAD = 0.10TL)

9. HOLE DIAMETER, GROUTED LENGTHS, DESIGN BOND STRESS TEST LOADS

TESTS FOR ROCK DOWELS GROUTED IN ROCK

- TEST HOLE DIAMETER = 4 INCHES
- GROUTED LENGTH = 8 FEET
- DESIGN BOND STRESS = 12.5 PSI
- FOR A 4-INCH DIAMETER DRILL HOLE, USE 1,885 LBS. PER LINEAL FOOT OF GROUTED NAIL

UNTIL STABLE

- LENGTH TO CALCULATE TL
- FOR A 8-FOOT GROUTED LENGTH, TL = 15.1 KIPS

10. DEFINITION AND PROCEDURES:

- PULLOUT DEFINITION: UNABLE TO SUSTAIN LOAD BECAUSE OF SIGNIFICANT BAR END MOVEMENT
- APPLY EACH TEST LOAD INCREMENT IN LESS THAN ONE MINUTE.
- HOLD EACH NON-CREEP TEST LOAD LONG ENOUGH TO OBTAIN STABLE LOAD/BAR-END DEFLECTION READINGS (IF NO PULLOUT)
- CREEP TESTS: PERFORM FITHER A 10-MINUTE OR A 60-MINUTE CREEP TEST AT THE CREEP TEST LOADS. START THE CREEP PERIOD (AND START RECORDING ELAPSED TIME) MIMEDIATELY AFTER APPLYING THE CREEP LOAD AND MAKE MEASUREMENTS AT ELAPSED TIMES OF 1, 2, 3, 5, 6 AND 10 MINUTES.
- IF THE DIFFERENTIAL MEASURED BAR-END DEFLECTIONS BETWEEN THE 1 MINUTE INCREMENT. AND THE 10 MINUTE INCREMENT IS MORE THAN 0.04-INCH, THEN:
- •• MAINTAIN THE LOAD
- CONTINUE RECORDING ELAPSED TIME
- •• CONTINUE THE CREEP TEST FOR AN ADDITIONAL 50 MINUTES
- •• MAKE MEASUREMENTS AT ELAPSED TIMES OF 15, 20, 25, 30, 45 AND 60 MINUTES
- TERMINATE THE CREEP TEST AND CONTINUE WITH THE NEXT LOAD INCREMENT
- IF THE DIFFERENTIAL MEASURED BAR-END DEFLECTIONS BETWEEN 1 MINUTE AND 10 MINUTE INCREMENTS IS 0.04-INCHES OR LESS, THEN TERMINATE THE CREEP TEST AND CONTINUE WITH THE NEXT LOAD INCREMENT.
- AFTER COMPLETING CREEP TESTS AT 2.0TL AND REDUCING THE LOAD TO AL (AND MAKING MEASUREMENTS AT THE AL), PERFORM THE VERIFICATION TEST
- VERIFICATION TEST: PULL TEST NAILS TO PULLOUT OR TO THE MAXIMUM BAR ALLOWABLE LOAD WHICHEVER IS LESS.
- THE CONTRACTOR MAY REMOVE THE BAR-END DEFLECTION DIAL-GAUGE BEFORE PERFORMING THE VERIFICATION TEST (TO AVOID DAMAGING THE GAUGE).
- DURING THE VERIFICATION TEST, MONITOR THE APPLIED LOAD (OR CHANGE IN THE APPLIED LOAD). IF THE BAR SUSTAINS THE MAXIMUM LOAD (WITH NO PULLOUT), THEN RECORD THE MAXIMUM LOAD, THE MAXIMUM BOND STRESS, AND THE CORRESPONDING BAR END DEFLECTION (IF THE DEFLECTION DIAL GAUGE IS STILL IN PLACE). IF THE DEFLECTION DIAL GAUGE IS STILL IN PLACE, THEN REDUCE THE LOAD TO THE AL AND MAKE/RECORD
- MEASURE AND RECORD THE LOAD AND THE BAR END DEFLECTION FOR EACH LOAD AND LOAD/TIME INCREMENT, INCLUDING THE AL INCREMENTS (AFTER APPLIED LOAD AND BAR-END MEASUREMENTS ARE STABLE). FOR THE CREEP TESTS, MEASURE AND RECORD THE ELAPSED TIME FOR EACH TIME INCREMENT. IF PULLOUT OCCURS, IDENTIFY/RECORD THE ATTEMPTED LOAD INCREMENT DURING WHICH PULLOUT OCCURS AND THE MAXIMUM LOAD AT PULLOUT.
- IF PULLOUT OCCURS AT A LOAD INCREMENT LESS THAN 1.5TL OR IF THE CREEP TEST AT 1.5TL DOES NOT MEET ACCEPTANCE CRITERIA. THEN REDUCE THE LOAD TO THE AL AND THEN PERFORM A CREEP TEST THAT MEETS ACCEPTANCE CRITERIA AT THE HIGHEST SUSTAINABLE
- THE ENGINEER SHALL RE-EVALUATE THE DESIGN IF ACCEPTANCE CRITERIA IS NOT MET.

11. ACCEPTANCE CRITERIA:

- TOTAL MOVEMENT OF NO MORE THAN 0.04 INCHES BETWEEN 1 MIN. AND 10 MIN. READINGS OR NO MORE THAN 0.08 INCHES BETWEEN 6 MIN. AND 60 MIN. READINGS DURING THE CREEP TEST AT 1.5TI
- BAR DEFLECTION AT 1.5TL > 80% OF THE THEORETICAL FLASTIC FLONGATION OF THE BAR LENGTH EXTENDING FROM THE TOP OF THE GROUT TO THE JACK NUT

INSPECTIONS TABLE

ITEM	TYPE (INSPECTION / TEST) BLANK IF NOT REQUIRED OR NOT APPLICABLE	DESCRIPTION (PERIODIC / CONTINUOUS INSPECTION OR TEST FREQUENCY)	REFERENCES FOR INSPECTION CODES AND TEST METHODS	NOTES
SITE PREPARATION	INSPECTION	PERIODIC	CBC 1705.6	
EARTHWORK	INSPECTION	PERIODIC	CBC 1705.6	
SHOTCRETE WALL BACKDRAINAGE	INSPECTION	PERIODIC		DESIGN REQUIREMENT
ROCK DOWEL DRILLING AND INSTALLATION	INSPECTION	PERIODIC	CBC 1705.8	
ROCK DOWEL GROUTING	TESTS	CONTINUOUS ONE CYLINDER SET PER SHIFT	CBC 1705.8, 1705.3 ASTM C31	
ROCK DOWEL GROUT	TESTS	EACH CYLINDER SET	ASTM C39	
ROCK DOWEL LOAD TESTS	INSPECTION	CONTINUOUS		DESIGN REQUIREMENTS
SHOTCRETE SHORING WALL SHOTCRETE	INSPECTION	PROPER MIX DESIGN VERIFICATION	CBC 1705.3	
SHOTCRETE SHORING WALL SHOTCRETE PLACEMENT	INSPECTION	CONTINUOUS ONE 20 X 20 X 5.5-INCH PANEL PER SHIFT (MAX. 50 CY PLACED) NOT INCLUDING BACKFILL SHOTCRETE	CBC 1705.3 ASTM C1140	
SHOTCRETE SHORING WALL SHOTCRETE	TESTS	EACH PANEL	ASTM C1604	
EPOXY ANCHORS	INSPECTION	PERIODIC	CBC 1705.3	





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LONG TUNNEL MAINTENANCE

JOAQUIN IRRIGATION AUS COUNTY, CALIFO SOUTH SAN J STANISL

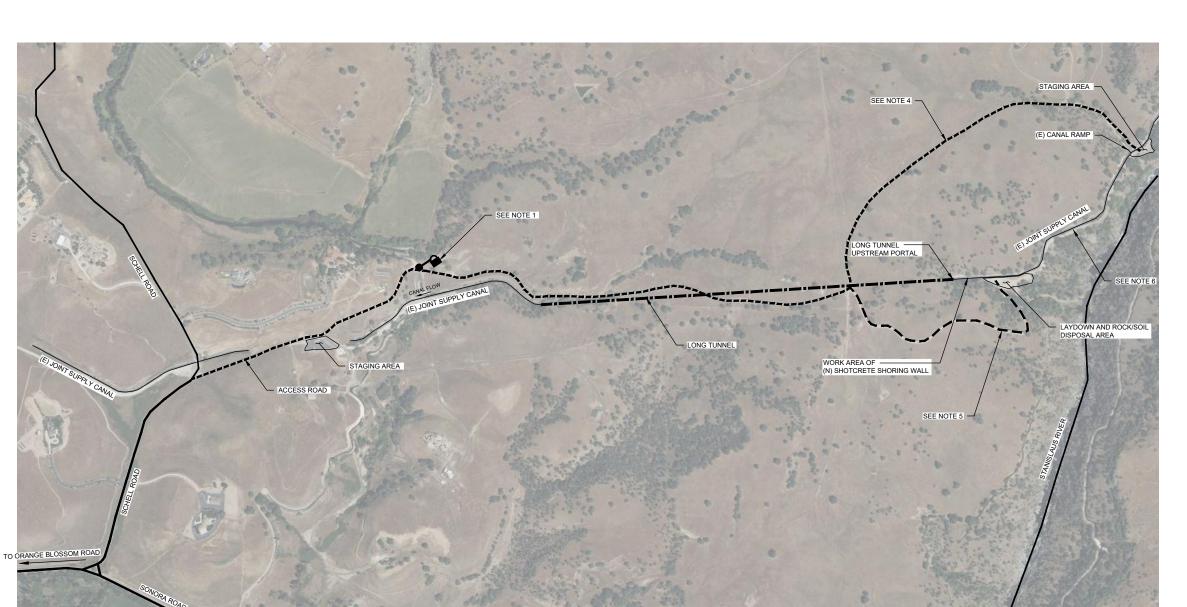
DESIGN ENGINEER: ANDREW KOSITSKY GF 2532

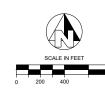
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ROJECT NO: 23 PHASE: 005

ORIGINAL SCALE SHOWN IS O INCH. ADJUST SCALE FOR REDUCED OR ENLARGED PLANS SHEET 1.1

2 OF





LEGEND

••

LOCKED GATE

SITE ACCESS ROADS

NOTES

- 1. SEE ACCESS NOTES ON SHEET 1.0.
- FINAL LOCATIONS AND SIZES OF LAYDOWN, STAGING, AND MATERIAL DISPOSAL AREAS SHALL BE APPROVED BY SSJID.
- 3. THE CONTRACTOR MUST ENSURE PROPERTY OWNER OPERATIONS ARE NOT IMPEDED DURING CONSTRUCTION. THIS INCLUDES BUT NOT LIMITED THE ABILITY TO MOVE AND OPERATE EQUIPMENT NEAR STAGING/LAYDOWN AREAS AND MAINTAIN ACCESS TO AND FROM CANAL BANK.
- 4. PRIMARY ALL-WEATHER SITE ACCESS ROAD.
- 5. SECONDARY FAIR-WEATHER SITE ACCESS ROAD.
- 6. LIMITED CANAL BANK ACCESS BETWEEN STAGING AREA AND LAYDOWN

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SOUTH SAN JOAQUIN IRRIGATION DISTRICT STANISLAUS COUNTY, CALIFORNIA

LONG TUNNEL MAINTENANCE

DESIGN ENGINEER: ANDREW KOSITSKY LICENSE NO: GE 2532

DRAFTED BY: CHECKED BY: KGM SWL

DATE: 09/06/2023 JOB NO: 1055-23005

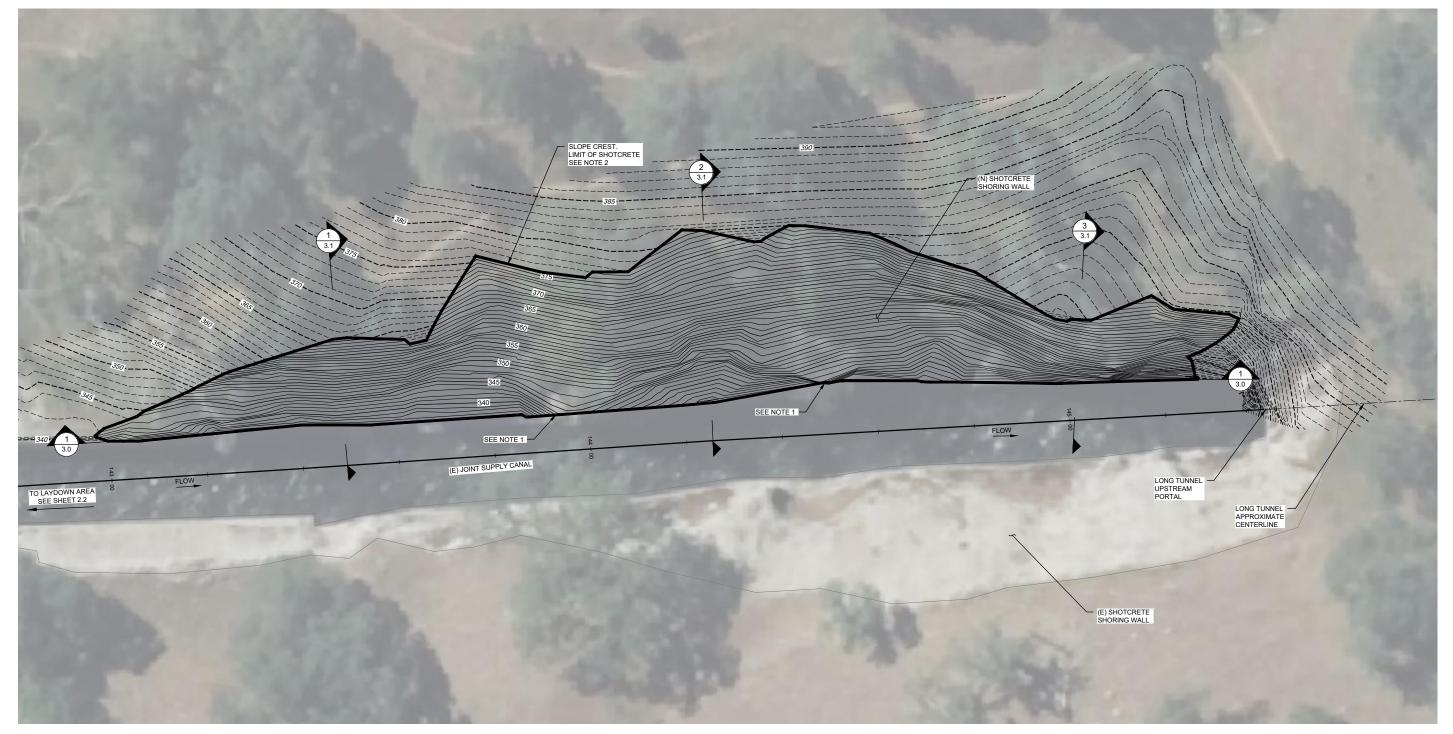
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ORIGINAL SCALE SHOWN IS ONE INCH. ADJUST SCALE FOR REDUCED OR ENLARGED PLANS. SHEET 2.0

DISCLAIMER: THIS PLAN REPRESENTS FEATURES FOR ILLUSTRATION PURPOSES ONLY. IT IS NOT A LEGAL SURVEY AND IS NOT INTENDED FOR USE IN DETERMINING BOUNDARIES OR DIMENSIONS. ANY USE OF THIS PLAN FOR PURPOSES OTHER THAN LOCATION OF FEATURES IS DONE SO AT THE USER'S RISK AND WITHOUT THE CONSENT OF PROVOST & PRITCHARD. SEE CIVIL DOCUMENTS FOR LOCATION OF BOUNDARIES AND FEATURES.





NOTES

- ACTUAL TOPOGRAPHY IN SOME AREAS VARIES FROM CONTOUR LINES AS SHOWN DUE TO GAPS IN SURVEY POINTS.
- 2. THE ACTUAL LIMITS OF NEW SHOTCRETE WILL BE DETERMINED BY THE ENGINEER DURING CONSTRUCTION.

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SOUTH SAN JOAQUIN IRRIGATION DISTRICT STANISLAUS COUNTY, CALIFORNIA LONG TUNNEL MAINTENANCE

DESIGN ENGINEER:
ANDREW KOSITSKY
LICENSE NO:
GE 2532
DRAFTED BY:
KGM
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SWL

DATE: 09/06/2023

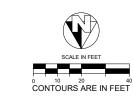
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NOTES

1. SEE NOTES ON SHEET 2.0.



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DESIGN ENGINEER:
ANDREW KOSITSKY
LICENSE NO:
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DRAFTED BY: CHECKED BY:
KGM SWL

DATE: 09/06/2023

JOB NO: 1055-23005

PROJECT NO: 23

PHASE: 005

ORIGINAL SCALE SHOWN IS ONE INCH. ADJUST SCALE FOR REDUCED OR ENLARGED PLANS. SHEET 2.2

DISCLAIMER: THIS PLAN REPRESENTS FEATURES FOR ILLUSTRATION PURPOSES ONLY. IT IS NOT A LEGAL SURVEY AND IS NOT INTENDED FOR USE IN DETERMINING BOUNDARIES OR DIMENSIONS. ANY USE OF THIS PLAN FOR PURPOSES OTHER THAN LOCATION OF FEATURES IS DONE SO AT THE USER'S RISK AND WITHOUT THE CONSENT OF PROVOST & PRITCHARD. SEE CIVIL DOCUMENTS FOR LOCATION OF BOUNDARIES AND FEATURES.

NOTES

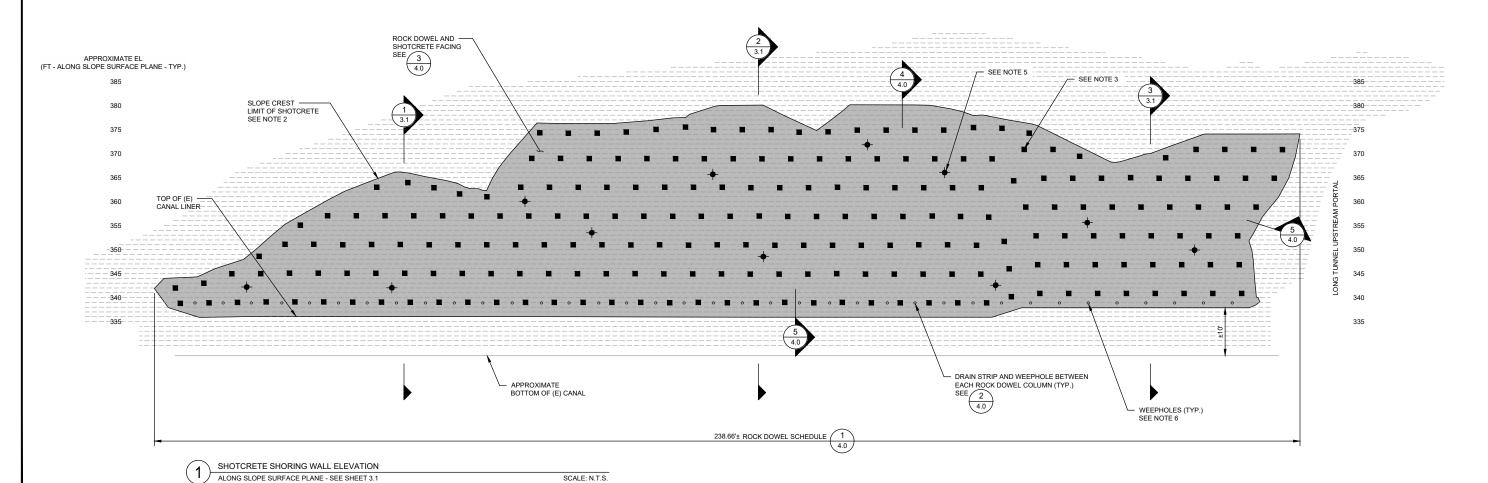
SHOTCRETE WALL APPROXIMATE AREA AS FOLLOWS FOR ESTIMATING PURPOSES:

SHORING WALL = ±9,882 SF

- THE ENGINEER SHALL CONFIRM THE ACUTAL LIMITS OF SHOTCRETE DURING CONSTRUCTION.
- LAYOUT OF ROCK DOWELS SHALL FOLLOW DESIGNATED SCHEDULE ON SHEET 4.0. PATTERNS SHOWN ON ELEVATIONS ARE FOR ILLUSTRATION PURPOSES ONLY.
- 4. DRAIN STRIPS ARE NOT SHOWN FOR CLARITY. SEE DETAIL 2 SHEET 4.0.
- 5. THE ENGINEER SHALL DETERMINE THE ACTUAL LOCATIONS OF TEST DOWEL DURING CONSTRUCTION BASED ON THEIR EVALUATION OF EXPOSED GROUND CONDITIONS. TEST DOWEL LOCATIONS ARE SHOWN FOR ILLUSTRATION AND PLANNING PURPOSES ONLY.
- PLACE WEEPHOLES ABOVE CANAL HIGH WATER LINE APPROXIMATELY 11 FEET ABOVE EXISTING CANAL INVERT.

LEGEND

- ROCK DOWEL
- → TEST DOWEL



LONG TUNNEL MAINTENANCE

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SOUTH SAN JOAQUIN IRRIGATION DISTRICT STANISLAUS COUNTY, CALIFORNIA

DESIGN ENGINEER: ANDREW KOSITSKY

GE 2532 DRAFTED BY: CHECKED BY

KGM SWL

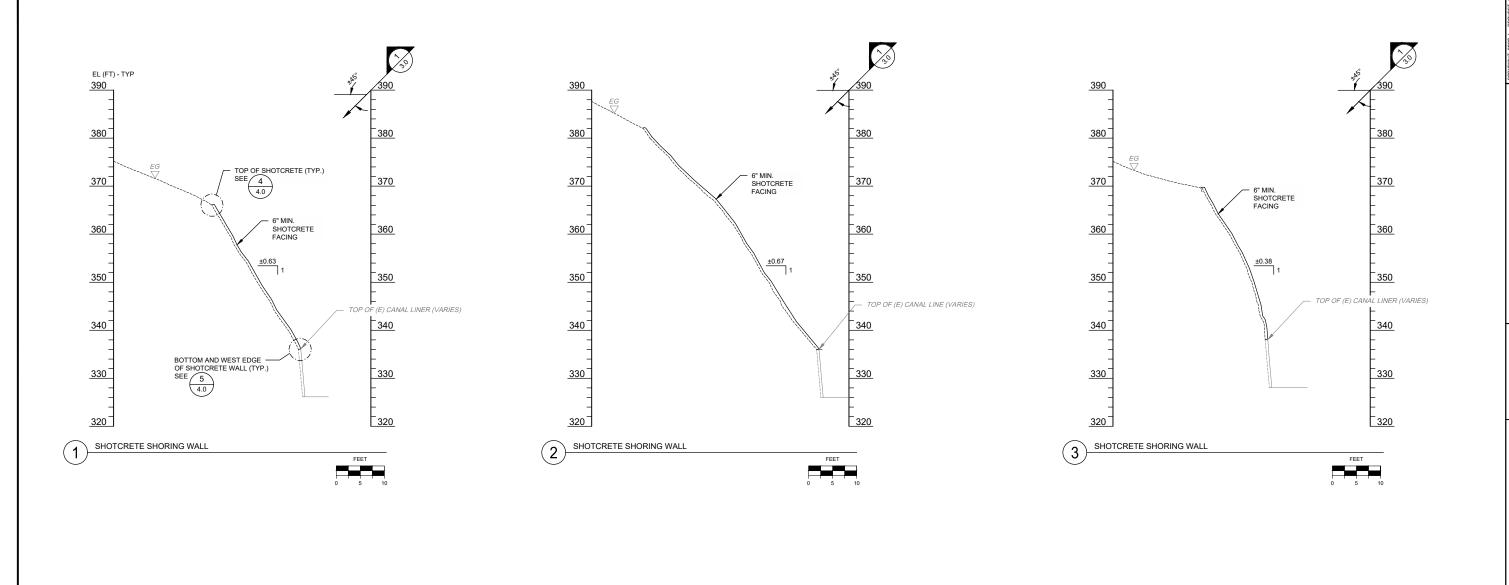
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3.0



SOUTH SAN JOAQUIN IRRIGATION DISTRICT STANISLAUS COUNTY, CALIFORNIA LONG TUNNEL MAINTENANCE

DESIGN ENGINEER:
ANDREW KOSITSKY
LICENSE NO:
GE 2532

DRAFTED BY: CHECKED BY: KGM SWL

JOB NO: 1055-23005

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SHEET 3.1

